List of Corrections in IS 800:2007 – Submitted for consideration at the meeting of BIS committee CED:7 on 1st October, 2010, at BIS, Chennai

No	Page	Clause/Table/Fig	Currently reads as	Change to
1	iii	Section **17 Fabrication and Erection**	(**17.15** missing)	**17.15** Bedding requirement
2	6	21st symbol explanation	C_{my}, C_{mz} – moment amplification factors about respective axes	K_z, K_y, K_{LT} – moment amplification factors (Cl. **4.4** and Cl. **9.3.2.2**)
3	18	Table 2: Web of an I, H or box section – Generally (under Class 1, Class 2 and Class 3)	but $\leq 42\varepsilon$	but $\geq 42\varepsilon$
4	18	Table 2 Ratio for stem of a T-section…	D/t_f	d/t
5	18	Table 2 Last line	overll	overall
6	19	Figure for ROLLED CHANNELS	Depth is marked as 'h'	Should be 'd'
7	24	Cl **4.4.2**, Cl **4.4.3.1**	C_y,C_z	K_y,K_z
8	24	Cl **4.4.3.3**	C_{my}, C_{mz}	K_y,K_z
9	25	Cl.**4.5.2** b) 12th line	IS 2062 shall not be than	IS 2062 shall not be **less** than
10	31	Cl **5.6.1** Deflection. 2nd column 6th line	Table 6 gives recommended limits of deflections for certain structural members and systems. Circumstances may arise….	Table 6 gives recommended limits of deflections for certain structural members and systems. In Table 6, Live load should include all post construction loads including super imposed dead loads (SIDL). Circumstances may arise….
11	33	Cl. **6.3.3** (max. value of beta)	$f_u\ \gamma_{mo}/f_y\ \gamma_{m1}$	$0.9\ f_u\ \gamma_{mo}/f_y\ \gamma_{m1}$
12	34	Cl. **7.1.2** First line	The design compressive strength P_d ,of a member is given by :	The factored design compression, P, in the members shall satisfy the following requirement:
13	34	Cl: **7.1.2.1** last line	λ_{m0}	γ_{m0}
14	35	Fig.8		Abscissa (X- axis) title "λ"
15	44	Table 10 Limits for Rolled I- sections	$40 \leq mm < t_f \leq 100mm$	$40\ mm < t_f \leq 100mm$
16	45	Table 11, Boundary at one end – 2nd Case	I Column II column Free Restrained	I Column II column Restrained Free
17	48	Cl. **7.5.1.2** end of the first para	λ_e as given below	λ_e as given below, in place of λ in **7.1.2.1** and use curve 'c' (α=0.49)

18	48	Cl. **7.5.1.2**	Rewrite the equations for λvv = …λφ=…	$\lambda_{vv} = \frac{\left(l/r_{vv}\right)}{\varepsilon\sqrt{\frac{\pi^2 E}{250}}}$ $\lambda_{\varphi} = \frac{(b_1 + b_2)/2t}{\varepsilon\sqrt{\frac{\pi^2 E}{250}}}$
19	49	Top figure in Fig.10		Correct the horizontal axis as z-z and vertical axis as y-y.
20	53	Cl. **8.2.1.1** 3rd line	d/tw ≤ 67ε	$d/t_w > 67\varepsilon$
21	57	Table 14 Heading	KL/r	L_{LT}/r_y
			h/t_f	h_f/t_f
22	58	1st para 3rd and 4th line	length for lateral torsional buckling to be used in **8.2.2.1** L_{LT} shall be	length for lateral torsional buckling, $L_{LT,}$ to be used in **8.2.2.1** shall be
23	58	1st para last three lines	The effective length shall be equal to 1.2 times the length of the relevant segment in between the lateral restraints	In the case of intermediate partial lateral restraints, the effective length, $L_{LT,}$ shall be taken as equal to 1.2 times the length of the relevant segment in between the partial lateral restraints
24	58	Cl. **8.3.2** 9th line	at the shear and is	at the shear centre and is
25	58	Table 15 Case iii) under warping restraint	Both flanges fully restrained	Both flanges partially restrained
26	59	Cl **8.4.2.1**	ε and f_y	Read all ε as ε_w and f_y as f_{yw}
27	60	Cl. **8.4.2.2** a) Definition of $\tau_{cr,e}$ = the elastic critical shear stress of the web	$t_{cr,e}$	$\tau_{cr,e}$
28	60	Second column second line	τ_b – buckling strength as obtained from 8.4.2.2.(a)	Repeated and hence treat that as deleted.
29	60	Cl.**8.5.1** – 3rd line	Carried in accordance	Carried out in accordance
30	60	Definition for ϕ	$= tan^{-1}(\frac{d}{c})$	= nearly equal to $\frac{\tan^{-1}\frac{d}{c}}{1.5}$
31	60	Cl **8.4.4.2**(b)	w_{tf}=dcosϕ+(c-s_c-s_t)sinϕ	w_{tf}=dcosϕ-(c-s_c-s_t)sinϕ
32	61	Table 16 (last row under top restraint conditions)	i) Free	The figure should be without top flange restraint
33	62	Fig. 12	Dimension arrow	The dimension is ‘c’
34	62	NOTES Item no 2 for	Panel B is designed	Panel B is designed using

		Figure 12	without utilizing the tension field action as given in **8.4.2.2.(a)**	simple post critical method as given in **8.4.2.2.(a)**
35	63	NOTES item no 2 for Figure 13		Treat item 2 as deleted and item 3 becomes item 2
36	63	Cl.**8.6.1.1** a) and b)	All occurrences of ε	ε_w
37	63	Cl.**8.6.1.1** b) 3)	When $c < d$	When $c < 0.74d$
38	64	Cl. **8.6.1.2** last line	ε_f = yield stress ratio of web	ε_f = yield stress ratio of flange
39	70	Cl**. 9.3.1.2** c) for standard I or H sections	for $n \leq 0.2$ $M_{ndy}=M_{dy}$ for $n > 0.2$ $M_{ndy}=1.56$ M_{dy} $(1-n)(n+0.6)$ $M_{ndz}=1.11M_{dz}(1-n) \leq M_{dz}$	$M_{ndz}=1.11$ M_{dz} $(1-n) \leq M_{dz}$ for $n \leq 0.2$ $M_{ndy}=M_{dy}$ for $n > 0.2$ $M_{ndy}=1.56$ M_{dy} $(1-n)(n+0.6)$
40	72	Table 18(4^{th} column, 6^{th} row)	$0.2(1-\psi)-0.8\alpha_s \geq 0.4$	$0.2(-\psi)-0.8\alpha_s \geq 0.4$
41	72	Table 18(2^{nd} column,6^{th} & 7^{th} rows)	α_s	α_h
42	72	Table 18(4^{th} column, 7^{th} & 8^{th} row)	0.095	0.95
43	72	Table 18 (5^{th} column, 9^{th} row)	$0.90+0.05\ \alpha_h(1+2\ \psi)$	$0.90+0.1\ \alpha_h(1+2\ \psi)$
44	75	Cl.**10.3.2**	$V_{sb} = V_{db}$	$V_{sb} \leq V_{db}$
45	76	Cl **10.4.3** Slip Resistance	Design for friction type bolting in which slip is required to be limited, a bolt subjected only to a factored…..	Design for friction type bolting where slip resistance is required at factored design force V_{sf} shall satify the following
46	76	Cl **10.4.3** definition of μ_f	$\mu_f=0.55$	$\mu_f \leq 0.55$
47	76	Cl.**10.4.3** NOTE:	V_{ns}	V_{nsf}
48	77	Page 77 6^{th} line	$0.9f_{ub}A_n \leq f_{yb}A_{sb}(\gamma_{m1}/\gamma_m)$	$0.9f_{ub}A_n \leq f_{yb}A_{sb}(\gamma_{m1}/\gamma_{m0})$
49	80	Cl. **10.5.10.2.2** second term in the equation under the square root sign	f_{bf}^2	f_{br}^2
50	89	Cl.**12.8.2.1** **12.11.1**	E250B steel of IS:2062 only	E250B steel of IS:2062 or steel meeting the requirement of Charpy E>27 J.
51	106	Cl.**16.4.1** value in denominator	905	690
52	121	B-3.2 equation and definitions of terms (add suffix i)	$\Phi_s = (\delta_u - \delta_{L)} / h$	$\Phi_{si} = (\delta_{ui} - \delta_{Li})/ h_i$
53	128	E-1.2 equation for Mcr	(L_{LT})	$(L_{LT})^2$

54	128	E-1.2 equation for yj	(z^2-y^2)	(z^2+y^2)
55	129	Definition I_t	I_t = torsion constant	I_t = St. Venant's torsion constant
56	130	Table 42 6th column 5th row	1.780	1.730
57	130	Table 42 6th column 7th row	1.257	1.267
58	130	Table 42 6th column 10th row	1.390	1.890